

FREE VIBRATION ANALYSIS OF RECTANGULAR PLATE BY SPLIT-DEFLECTION METHOD

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Abstract

This paper presents free vibration analysis of rectangular plate by split-deflection method. In this method, the deflection was split into x and y components of deflection. That is the deflection of the rectangular plate was taken as the product of these two components. Having made this assumption, the study went ahead to formulate total potential energy functional from principles of theory of elasticity based on work-error approach. This energy functional was minimized by direct variation and equation for resonating frequency was obtained. Two illustrative examples were used to test this method. They are plates with all edges simply supported and all edges clamped. The first case used polynomial function for x component of deflection and trigonometric function for y component of. However, the second example used polynomial function for both x and y components of deflection. Fundamental resonating frequencies (in non dimensional forms) of the two plates for aspect ratios ranging from 1.0 to 2.0 (at increment of 0.1) were determined and compared with the values from previous study. From the comparison, it was observed that the maximum percentage difference of 0.06 was recorded for the first example at aspect ratios of 1.1 and 1.3 with non dimensional resonating frequencies of 18.03 and 15.71 respectively. For the

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second example, the maximum percentage difference of 0.09 was recorded at aspect ratio of 1.1 with non dimensional resonating frequencies of 32.96. This small value of percentage differences show that this present method is adequate and reliable for classical plate theory (CPT) free vibration analysis of rectangular plates.

Index Terms—Resonating frequency, split-deflection, work-error, energy functional, polynomial function, trigonometric function

I. INTRODUCTION

Most energy methods used for classical plate theory (CPT) free vibration analysis of rectangular plates include Raleigh, Raleigh-Ritz, Ritz, Galerkin, minimum potential energy, work-error etc (Ugural, 1999, Ventsel and Krauthammer, 2001 and Ibearugbulem et al., 2014). The deflection (displacement normal to the plane of the plate is a single orthogonal function, w). This is apparent in the energy functional for Raleigh, Raleigh-Ritz and Ritz. Typical Ritz energy functional is (Ibearugbulem et al., 2014):

$$\Pi = \frac{D}{2} \int_0^a \int_0^b \left(\left[\frac{\partial^2 w}{\partial x^2} \right]^2 + 2 \left[\frac{\partial^2 w}{\partial x \partial y} \right]^2 + \left[\frac{\partial^2 w}{\partial y^2} \right]^2 \right) \partial x \partial y - \frac{m \cdot \lambda^2}{2} \int_0^a \int_0^b w^2 \partial x \partial y$$

The use of single orthogonal deflection function is also seen in Galerkin and work-error methods. Typical work-error functional is (Ibearugbulem et al., 2014):

$$\Pi = \frac{D}{2} \int_0^a \int_0^b \left(\frac{\partial^4 w}{\partial x^4} w + 2 \frac{\partial^4 w}{\partial x^2 \partial y^2} w + \frac{\partial^4 w}{\partial y^4} w \right) \partial x \partial y - \frac{m \cdot \lambda^2}{2} \int_0^a \int_0^b w^2 \partial x \partial y$$

From the literature so far, most scholarly works on CPT analysis of rectangular plates rely on this single orthogonal function (Hutchinson, 1992, Jianqiao, 1994, Ugural, 1999, Ventsel and Krauthammer, 2001, Wang et al., 2002, Taylor and Govindjee, 2004, Szilard, 2004, Jiu et al., 2007, Erdem et al., 2007, Ezeh et al., 2013, Ibearugbulem, 2014). Obviously, one can assert that all energy functional in use are based on single orthogonal deflection function and none has used a

deflections function that is typically separated into two independent distinct functions ($w = w_x * w_y$). In this present study w_x and w_y may be both polynomials or trigonometric functions or w_x may be polynomials while w_y may be trigonometry. The main reason for this modification is to help the analysis who may have difficulty in obtaining orthogonal function for a plate of a particular boundary condition. In this case, the analyst who may have easy access to deflection equations for beams of any boundary condition can find the proposed method quite useful and handy.

II. BASIC ASSUMPTIONS

The assumption here is that the general deflection, w is split into w_x and w_y . That is the split-deflection function is given as:

$$w = w_x \cdot w_y \quad 1$$

Where the w_x and w_y components of the deflection are defined as:

$$w_x = \sqrt{A} \cdot h_1 \quad 2$$

$$w_y = \sqrt{A} \cdot h_2 \quad 3$$

Substituting equations (2) and (3) into equation (1) gives:

$$w = A h_1 h_2 \quad 4$$

III. IN-PLANE DISPLACEMENTS

From the assumption that vertical shear strains are zero for classical plate and making use of split-deflection, we obtain:

$$u = -z \frac{dw}{dx} = -z \frac{dw_x}{dx} w_y \quad 5$$

$$v = -z \frac{dw}{dy} = -z \frac{dw_y}{dy} w_x \quad 6$$

IV. STRAIN DEFLECTION RELATIONSHIP

Differentiating equations (5) and (6), the three in-plane strains of CPT are given as:

$$\varepsilon_x = \frac{du}{dx} = -z \frac{d^2 w_x}{dx^2} w_y \quad 7$$

$$\varepsilon_y = \frac{dv}{dy} = -z \frac{d^2 w_y}{dy^2} w_x \quad 8$$

$$\gamma_{xy} = \frac{du}{dy} + \frac{dv}{dx} = -2z \frac{dw_x}{dx} \frac{dw_y}{dy} \quad 9$$

V. STRESS – STRAIN RELATIONSHIP

The CPT constitutive equations for plane stress plate are given as:

$$\sigma_x = \frac{E}{1 - \mu^2} [\varepsilon_x + \mu\varepsilon_y] \quad 10$$

$$\sigma_y = \frac{E}{1 - \mu^2} [\mu\varepsilon_x + \varepsilon_y] \quad 11$$

$$\tau_{xy} = \frac{E(1 - \mu)}{2(1 - \mu^2)} \gamma_{xy} \quad 12$$

VI. STRESS – DEFLECTION RELATIONSHIP

Substituting equations (7), (8) and (9) into equations (10), (11) and (12) where appropriate gives the split-deflection stress-deflection equation as:

$$\sigma_x = \frac{-Ez}{1 - \mu^2} \left[\frac{d^2 w_x}{dx^2} w_y + \mu \frac{d^2 w_y}{dy^2} w_x \right] \quad 13$$

$$\sigma_y = \frac{-Ez}{1 - \mu^2} \left[\mu \frac{d^2 w_x}{dx^2} w_y + \frac{d^2 w_y}{dy^2} w_x \right] \quad 14$$

$$\tau_{xy} = \frac{-Ez(1 - \mu)}{(1 - \mu^2)} \frac{dw_x}{dx} \frac{dw_y}{dy} \quad 15$$

VII. TOTAL POTENTIAL ENERGY

The strain energy is defined as:

$$U = \frac{1}{2} \int_x \int_y \left[\int_{-\frac{t}{2}}^{\frac{t}{2}} [\sigma_x \varepsilon_x + \sigma_y \varepsilon_y + \tau_{xy} \varepsilon_{xy}] dz \right] dx dy \quad 16$$

For pure bending analysis, the external work is given as:

$$V = \int_x \int_y \frac{m \cdot \lambda^2}{2} w_x^2 \cdot w_y^2 dx dy$$

That is

$$V = \frac{m \cdot \lambda^2}{2} \int_x w_x^2 dx \int_y w_y^2 dy \quad 17$$

Substituting equations (10) to (15) into equation (16) gives strain energy – deflection relationship as:

$$U = \frac{D}{2} \int_x \int_y \left[\left(\frac{d^2 w_x}{dx^2} \right)^2 w_y^2 + 2 \left(\frac{dw_x}{dx} \right)^2 \left(\frac{dw_y}{dy} \right)^2 + \left(\frac{d^2 w_y}{dy^2} \right)^2 w_x^2 \right] dx dy$$

That is in the work-error approach, the strain energy becomes:

$$\begin{aligned} U &= \frac{D}{2} \left[\int_x \frac{d^4 w_x}{dx^4} w_x dx \int_y w_y^2 dy \right] \\ &+ \frac{2D}{2} \left[\int_x \frac{d^2 w_x}{dx^2} w_x dx \int_y \frac{d^2 w_y}{dy^2} w_y dy \right] \\ &+ \frac{D}{2} \left[\int_x w_x^2 dx \int_y \frac{d^4 w_y}{dy^4} w_y dy \right] \end{aligned} \quad 18$$

Subtracting equation (17) from Equation (18) gives the total potential energy functional as:

$$\begin{aligned} \Pi &= \frac{D}{2} \left[\int_x \frac{d^4 w_x}{dx^4} w_x dx \int_y w_y^2 dy \right] \\ &+ \frac{2D}{2} \left[\int_x \frac{d^2 w_x}{dx^2} w_x dx \int_y \frac{d^2 w_y}{dy^2} w_y dy \right] \\ &+ \frac{D}{2} \left[\int_x w_x^2 dx \int_y \frac{d^4 w_y}{dy^4} w_y dy \right] - \frac{m \cdot \lambda^2}{2} \int_x w_x^2 dx \int_y w_y^2 dy \end{aligned} \quad 19$$

Substituting equations (1) and (2) into equation (19) gives:

$$\begin{aligned} \Pi &= \frac{A^2 D}{2} \left[\int_x \frac{d^4 h_1}{dx^4} h_1 dx \int_y h_2^2 dy \right] \\ &+ \frac{2A^2 D}{2} \left[\int_x \frac{d^2 h_1}{dx^2} h_1 dx \int_y \frac{d^2 h_2}{dy^2} h_2 dy \right] \\ &+ \frac{A^2 D}{2} \left[\int_x h_1^2 dx \int_y \frac{d^4 h_2}{dy^4} h_2 dy \right] \\ &- \frac{m \cdot \lambda^2}{2} A^2 \int_x h_1^2 dx \int_y h_2^2 dy \end{aligned} \quad 20$$

Now, equation (20) can be written in non dimensional axes R and Q.

$$x = aR \quad 21$$

$$y = aQ \quad 22$$

$$P = b/a \quad 23$$

Where a, b and P are the plate lengths in x and y axes and long span- short span aspect ratio respectively.

Substituting equations (21), (22) and (23) into equation (20) gives:

$$\begin{aligned} \Pi = & \frac{abA^2D}{2a^4} \left[\int_0^1 \frac{d^4h_1}{dR^4} h_1 dR \int_0^1 h_2^2 dQ \right] \\ & + 2 \frac{abA^2D}{2a^4P^2} \left[\int_0^1 \frac{d^2h_1}{dR^2} h_1 dR \int_0^1 \frac{d^2h_2}{dQ^2} h_2 dQ \right] + \frac{abA^2D}{2a^4P^4} \left[\int_0^1 h_1^2 dR \int_0^1 \frac{d^4h_2}{dQ^4} h_2 dQ \right] \\ & - \frac{m \cdot \lambda^2}{2} A^2 ab \int_0^1 h_1^2 dR \int_0^1 h_2^2 dQ \quad 24 \end{aligned}$$

VIII. DIRECT VARIATION OF TOTAL POTENTIAL ENERGY

Equation (24) shall be differentiated with respect to the deflection coefficient, A and the result is:

$$\begin{aligned} \frac{d\Pi}{dA} = & \frac{AD}{a^4} \left[\int_0^1 \frac{d^4h_1}{dR^4} h_1 dR \int_0^1 h_2^2 dQ \right] \\ & + 2 \frac{AD}{a^4P^2} \left[\int_0^1 \frac{d^2h_1}{dR^2} h_1 dR \int_0^1 \frac{d^2h_2}{dQ^2} h_2 dQ \right] \\ & + \frac{AD}{a^4P^4} \left[\int_0^1 h_1^2 dR \int_0^1 \frac{d^4h_2}{dQ^4} h_2 dQ \right] \\ & - m \cdot \lambda^2 A \int_0^1 h_1^2 dR \int_0^1 h_2^2 dQ = 0 \end{aligned}$$

That is

$$\begin{aligned} \frac{D}{a^4} \left[\int_0^1 \frac{d^4h_1}{dR^4} h_1 dR \int_0^1 h_2^2 dQ \right] \\ & + 2 \frac{D}{a^4P^2} \left[\int_0^1 \frac{d^2h_1}{dR^2} h_1 dR \int_0^1 \frac{d^2h_2}{dQ^2} h_2 dQ \right] \\ & + \frac{D}{a^4P^4} \left[\int_0^1 h_1^2 dR \int_0^1 \frac{d^4h_2}{dQ^4} h_2 dQ \right] = m \cdot \lambda^2 \int_0^1 h_1^2 dR \int_0^1 h_2^2 dQ \quad 25 \end{aligned}$$

This equation (25) is the direct governing equation of rectangular plate under free vibration using work-error approach from this present method. Rearranging equation (25) and making resonating frequency, λ the subject of the equation gives:

$$\lambda^2 = \left(\frac{k_x + 2 \frac{k_{xy}}{p^2} + \frac{k_y}{p^4}}{k_\lambda} \right) * \frac{D}{ma^4} \quad 26$$

Where

$$k_x = \int_0^1 \frac{d^4 h_1}{dR^4} h_1 dR \int_0^1 h_2^2 dQ \quad 27$$

$$k_{xy} = \int_0^1 \frac{d^2 h_1}{dR^2} h_1 dR \int_0^1 \frac{d^2 h_2}{dQ^2} h_2 dQ \quad 28$$

$$k_y = \int_0^1 h_1^2 dR \int_0^1 \frac{d^4 h_2}{dQ^4} h_2 dQ \quad 29$$

$$k_\lambda = \int_0^1 h_1^2 dR \int_0^1 h_2^2 dQ \quad 30$$

IX. NUMERICAL EXAMPLE

Analyze a classical rectangular thin isotropic plate with:

i all the four edge simply supported using polynomial and trigonometry functions respectively for w_x and w_y .

ii all the four edge clamped using only polynomial function for both w_x and w_y

All Simple supported edge rectangular plate

$$w_x = \sqrt{A} (R - 2R^3 + R^4) \quad 31$$

$$w_y = \sqrt{A} \sin \pi Q \quad 32$$

From equations (31) and (32), h_1 and h_2 are:

$$h_1 = R - 2R^3 + R^4 \quad 33$$

$$h_2 = \sin \pi Q \quad 34$$

With these we obtain:

$$\int_0^1 h_1^2 dR = \frac{31}{630} \text{ and } \int_0^1 h_2^2 dQ = 0.5.$$

$$\int_0^1 \frac{d^4 h_1}{dR^4} h_1 dR = 4.8 \text{ and } \int_0^1 \frac{d^4 h_2}{dQ^4} h_2 dQ = 0.5\pi^4$$

$$\int_0^1 \frac{d^2 h_1}{dR^2} h_1 dR = \frac{17}{35} \text{ and } \int_0^1 \frac{d^2 h_2}{dQ^2} h_2 dQ = 0.5\pi^2.$$

$$k_x = (4.8)(0.5) = 2.4 \quad 35$$

$$k_{xy} = \left(\frac{17}{35}\right)(0.5\pi^2) = 2.3969 \quad 36$$

$$k_y = \left(\frac{31}{630}\right)(0.5\pi^4) = 2.3966 \quad 37$$

$$k_\lambda = \left(\frac{31}{630}\right)(0.5) = 0.02460315 \quad 38$$

Substituting equations (35) to (38) into equation (26) gives

$$\lambda^2 = \left(\frac{2.4 + \frac{4.7938}{P^2} + \frac{2.3966}{P^4}}{\frac{31}{1260}} \right) * \frac{D}{ma^4}. \text{ That is}$$

$$\lambda^2 = \left(97.5484 + \frac{194.8448}{P^2} + \frac{97.4102}{P^4} \right) * \frac{D}{ma^4} \quad 39$$

All clamped edge rectangular plate

$$w_x = \sqrt{A} (R^2 - 2R^3 + R^4) \quad 40$$

$$w_y = \sqrt{A} (Q^2 - 2Q^3 + Q^4) \quad 41$$

From equations (40) and (41), h_1 and h_2 are:

$$h_1 = R^2 - 2R^3 + R^4 \quad 42$$

$$h_2 = Q^2 - 2Q^3 + Q^4 \quad 43$$

With these we obtain:

$$\int_0^1 h_1^2 dR = \frac{1}{630} \text{ and } \int_0^1 h_2^2 dQ = \frac{1}{630}.$$

$$\int_0^1 \frac{d^4 h_1}{dR^4} h_1 dR = 0.8 \text{ and } \int_0^1 \frac{d^4 h_2}{dQ^4} h_2 dQ = 0.8$$

$$\int_0^1 \frac{d^2 h_1}{dR^2} h_1 dR = \frac{2}{105} \text{ and } \int_0^1 \frac{d^2 h_2}{dQ^2} h_2 dQ = \frac{2}{105}$$

$$k_x = (0.8) \left(\frac{1}{630} \right) = \frac{2}{1575} \quad 44$$

$$k_{xy} = \left(\frac{2}{105} \right) \left(\frac{2}{105} \right) = \frac{4}{11025} \quad 45$$

$$k_y = \left(\frac{1}{630} \right) (0.8) = \frac{2}{1575} \quad 46$$

$$k_\lambda = \left(\frac{1}{630} \right) \left(\frac{1}{630} \right) = \frac{1}{396900} \quad 47$$

Substituting equations (44) to (47) into equation (26) gives

$$\lambda^2 = \left(\frac{\frac{2}{1575} + \frac{8}{11025P^2} + \frac{2}{1575P^4}}{\frac{1}{396900}} \right) * \frac{D}{ma^4}. \text{ That is}$$

$$\lambda^2 = \left(504 + \frac{288}{P^2} + \frac{504}{P^4} \right) * \frac{D}{ma^4} \quad 48$$

RESULTS AND CONCLUSIONS

The non dimensional form of the resonating frequencies for different aspect ratios for ssss and cccc plates are shown on tables 1 and 2. A close critical examination of the tables reveals that the maximum percentage difference between the values from the present study and those from previous study is 0.09. From statistical point of view, this implies that no difference existed between the two sets of values. Thus, one can infer that the procedure, the deflection function and the energy functional formulated in this present study are reliable and adequate in CPT free vibration analysis rectangular plates. Hence, this method is recommended for stability analysis of CPT plates. It is also recommended that the present method is extended to refined plate theory analysis (RPT).

Table 1: Non dimensional form of resonating frequency of ssss isotropic thin plate

Aspect ratio, P	Resonating frequency, $\lambda \left(\frac{1}{a^2} \sqrt{\frac{D}{m}} \right)$		Percentage difference
	Present	Past (Ibearugbulem et al., 2014)	

1	19.74	19.75	-0.05
1.1	18.03	18.04	-0.06
1.2	16.73	16.73	0.00
1.3	15.71	15.72	-0.06
1.4	14.91	14.91	0.00
1.5	14.26	14.26	0.00
1.6	13.73	13.73	0.00
1.7	13.29	13.29	0.00
1.8	12.92	12.92	0.00
1.9	12.61	12.61	0.00
2	12.34	12.34	0.00

Table 2: Non dimensional form of resonating frequency of cccc isotropic thin plate

Aspect ratio, P	Resonating frequency, $\lambda\left(\frac{1}{a^2}\sqrt{\frac{D}{m}}\right)$		Percentage difference
	Present	Past (Ibearugbulem et al., 2014)	
1	36	35.97	0.08
1.1	32.96	32.93	0.09
1.2	30.77	30.75	0.07
1.3	29.17	29.15	0.07
1.4	27.97	27.95	0.07
1.5	27.05	27.03	0.07
1.6	26.33	26.31	0.08
1.7	25.77	25.75	0.08
1.8	25.32	25.30	0.08
1.9	24.95	24.94	0.04
2	24.65	24.64	0.04

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